



Approval body for construction products and types of construction

Bautechnisches Prüfamt

An institution established by the Federal and Laender Governments



European Technical Assessment

General Part

Technical Assessment Body issuing the European Technical Assessment:

Trade name of the construction product

Product family to which the construction product belongs

Manufacturer

Manufacturing plant

This European Technical Assessment contains

This European Technical Assessment is issued in accordance with Regulation (EU) No 305/2011, on the basis of

ETA-11/0319 of 17 July 2014

Deutsches Institut für Bautechnik

Tecfi wedge Anchor AJE

Torque controlled expansion anchor of sizes M8, M10, M12, M16 and M20 for use in concrete

Tecfi S.p.A Strada Statale Appia, Km. 193 81050 PASTORANO (CE) ITALIEN

tecfi plant

14 pages including 10 annexes which form an integral part of this assessment

Guideline for European technical approval of "Metal anchors for use in concrete", ETAG 001 Part 2: "Torque controlled expansion anchors", April 2013, used as European Assessment Document (EAD) according to Article 66 Paragraph 3 of Regulation (EU) No 305/2011.

ETA-11/0319 issued on 1 November 2012



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Specific Part

1 Technical description of the product

The Tecfi Wedge anchor AJE is an anchor made of galvanised steel of sizes M8, M10, M12, M16 and M20 which is placed into a drilled hole and anchored by torque-controlled expansion. The product description is given in Annex A.

2 Specification of the intended use in accordance with the applicable European Assessment Document

The performances given in Section 3 are only valid if the anchor is used in compliance with the specifications and conditions given in Annex B.

The verifications and assessment methods on which this European Technical Assessment is based lead to the assumption of a working life of the anchor of at least 50 years. The indications given on the working life cannot be interpreted as a guarantee given by the producer, but are to be regarded only as a means for choosing the right products in relation to the expected economically reasonable working life of the works.

3 Performance of the product and references to the methods used for its assessment

3.1 Mechanical resistance and stability (BWR 1)

Essential characteristic	Performance
Characteristic resistance for tension loads	See Annex C 1
Characteristic resistance for shear loads	See Annex C 1
Displacements under tension loads	See Annex C 3
Displacements under shear loads	See Annex C 3

3.2 Safety in case of fire (BWR 2)

Essential characteristic	Performance
Reaction to fire	Anchorages satisfy requirements for Class A1
Resistance to fire	See Annex C 2

3.3 Hygiene, health and the environment (BWR 3)

Regarding dangerous substances there may be requirements (e.g. transposed European legislation and national laws, regulations and administrative provisions) applicable to the products falling within the scope of this European Technical Assessment. In order to meet the provisions of Regulation (EU) No 305/2011, these requirements need also to be complied with, when and where they apply.



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3.4 Safety in use (BWR 4)

The essential characteristics regarding Safety in use are included under the Basic Works Requirement Mechanical resistance and stability.

- 3.5 Protection against noise (BWR 5) Not applicable.
- 3.6 Energy economy and heat retention (BWR 6) Not applicable.

3.7 Sustainable use of natural resources (BWR 7)

The sustainable use of natural resources was not investigated.

3.8 General aspects

The verification of durability is part of testing the essential characteristics. Durability is only ensured if the specifications of intended use according to Annex B are taken into account.

4 Assessment and verification of constancy of performance (AVCP) system applied, with reference to its legal base

According to Decision of the Commission of 24 June 1996 (96/582/EC) (OJ L 254 of 08.10.96 p. 62-65), the system of assessment and verification of constancy of performance (see Annex V and Article 65 Paragraph 2 to Regulation (EU) No 305/2011) given in the following table applies.

Product	Intended use	Level or class	System
Metal anchors for use in concrete (heavy-duty type)	For fixing and/or supporting concrete structural elements or heavy units such as cladding and suspended ceilings	_	1

5 Technical details necessary for the implementation of the AVCP system, as provided for in the applicable European Assessment Dcoument

Technical details necessary for the implementation of the AVCP system are laid down in the control plan deposited at Deutsches Institut für Bautechnik.

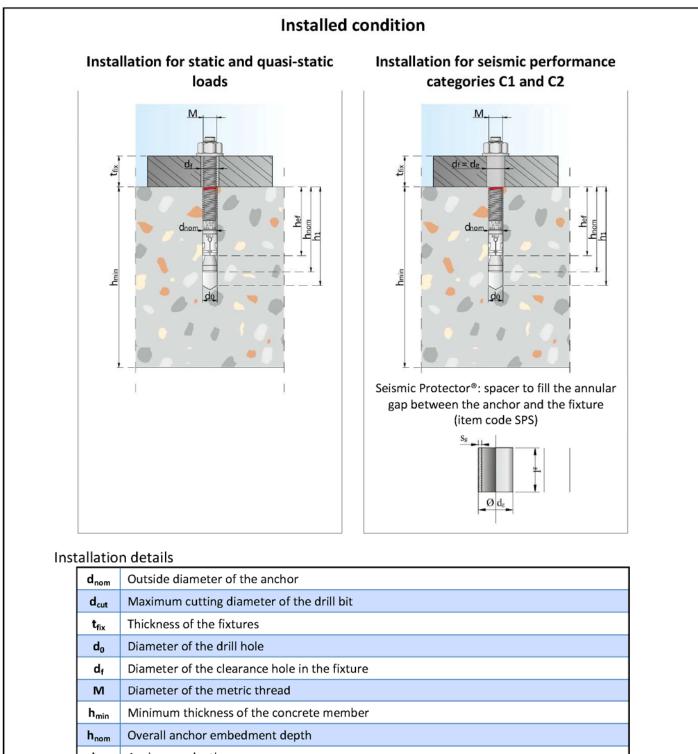
Issued in Berlin on 21 July 2014 by Deutsches Institut für Bautechnik

Uwe Bender Head of Department *beglaubigt:* Lange

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- h_{ef} Anchorage depth
- d_g Diameter of the spacer
- Ig Length of the spacer
- s_g Thickness of the spacer

Tecfi wedge anchor AJE

Product description Installed condition Annex A 1

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Anchor types					
(H) Letter		Letter of the thickn see ta	Anch of fix Anch XX" Mark ember Code on t cone bolt ess of fixt ble B.2	edment depth he head (maxium ure),	
	product '	"T-AJE" (on the co the clip)	ne bolt or	on	
AJE 01	components:	A	JE 31 c	components:	
Part	Description		Part	Description	1
1	Sleeve expansion		1	Sleeve expansion	
2	ISO 7089 regular washer		2	ISO 7093-1 large was	ner
3	Hexagonal nut		3	Hexagonal nut	
4	Cone bolt		4	Cone bolt	
SPS – S Part	Description	seismic perfo	rmance	categories C1 and	C2
Tecfi wedge a	ion				Annex A 2
Anchor types and	i components				

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Table A1: Materials and components AJE 01 - AJE 31 components Part Description Component **Cone Bolt** Zinc plated min 5 μ m (Cr VI Free) according to ISO 4042 carbon 1 steel cone bolt, minimum tensile strength 800 N/mm² **Hexagonal nut** Zinc plated min 5 µm (Cr VI Free) according to ISO 4042 carbon 2 steel hexagonal nut DIN 934 (or ISO 4032). Washer Zinc plated min 5 μ m (Cr VI Free) according to ISO 4042 carbon 3 steel washer ISO 7089 (AJE01) or ISO 7093-1 (AJE31), hardness class HV 200. Steel sleeve Zinc plated min 5 µm (Cr VI Free) according to ISO 4042 carbon 4 steel HRB 80. SPS components Zinc plated min 5 µm (Cr VI Free) according to ISO 4042 Seismic Protector[®], spacer for seismic performance 1 carbon steel spacer(s). categories C1 and C2 Assembled anchor For static and quasi-static laods AJE 01 AJE 31 For seismic performance categories C1 and C2 AJE 01 + SPS AJE 31 + SPS Tecfi wedge anchor AJE Annex A 3 **Product description** Materials

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Specifications of intended use

Anchorages subject to:

- Static and quasi-static loads: M8, M10, M12, M16, M20
- Seismic action for Performance Category C1 and C2: sizes M10, M12, M16, M20 with Seismic Protector® only
- Fire exposure: up to 120 minutes: M8, M10, M12, M16, M20

Base materials:

- Reinforced or unreinforced normal weight concrete according to EN 206-1:2000-12.
- Strength classes C20/25 to C50/60 according to EN 206-1:2000-12. (e.g.)
- Non-cracked concrete: M8, M10, M12, M16, M20
- Cracked concrete: M8, M10, M12, M16, M20.

Use conditions (Environmental conditions):

· Anchorages subject to dry internal conditions

Design:

- Anchorages are designed under the responsibility of an engineer experienced in anchorages and concrete work.
- Verifiable calculation notes and drawings are prepared taking account of the loads to be anchored. The position of the anchor is indicated on the design drawings (e.g. position of the anchor relative to reinforcement or to supports, etc.).
- · Anchorages under static or quasi-static actions and under fire exposure are designed in accordance with:
 - ETAG 001, Annex C, design method A, Edition August 2010;
 - CEN TS CEN/TS 1992-4-1:2009;
- Anchorages under seismic actions are designed in accordance with:
 - EOTA Technical Report TR 045, Edition February 2013
 - Anchorages shall be positioned outside of critical regions (e.g. plastic hinges) of the concrete structure.
 - Fastenings in stand-off installation or with a grout layer are not allowed
- In case of requirements for resistance to fire exposure it must be ensured that local spalling of the concrete cover does not occur.

Installation:

- Hole drilling by rotary plus hammer mode: M8, M10, M12, M16, M20
- Anchor installation carried out by appropriately qualified personnel and under the supervision of the person responsible for technical matters of the site.
- In case of aborted hole: new drilling at a minimum distance away of twice the depth of the aborted hole or smaller distance if the aborted hole is filled with high strength mortar and if under shear or oblique tension load it is not the direction of the load application.

Tecfi wedge anchor AJE

Intended Use Specifications Annex B 1

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Anchor size			M 8	M 10	M 12	M 16	M 20
Nominal drill hole diameter	do	[mm]	8	10	12	16	20
Maximum cutting diameter of drill bit	d_{cut}	[mm]	8,45	10,45	12,5	16,5	20,55
Maximum torque moment	T _{inst}	[Nm]	20	45	60	110	200
Minimum allowable spacing (even in case of fire exposure)	S _{min}	[mm]	80	65	75	130	170
Minimum allowable edge distance	C _{min}	[mm]	80	80	90	130	200
Wrench size	SW	[mm]	13	17	19	24	30
Overall anchor embedment depth	h _{nom}	[mm]	55	70	85	100	115
Minimum thickness of concrete member	h _{min}	[mm]	100	110	140	170	200
Depth of the drilled hole to deepest point	h1	[mm]	65	85	105	120	135
Diameter of clearance hole in the fixture	d _f	[mm]	9	12	14	18	22
Thickness of fixture	t _{fix}	[mm]	≤ 160	≤ 160	≤ 270	≤ 320	
Nominal outside diameter of the spacer for seismic performance categories C1 and C2	dg	[mm]	NPD	12	14	18	22
Nominal length of the spacer for seismic performance categories C1 and C2	lg	[mm]	NPD	The total length of the spacer must be equal to the thickness of the fixture, wit tolerance of: - for $t_{fix} \le 120$ [mm]: + 0 - 3 [mm] - for $t_{fix} > 120$ [mm]: + 0 - 5 [mm] More spacers can be used to reach the total length			
Minimum edge distance (fire exposure on one side)	C _{min}	[mm]			2 h _{ef}		
Minimum edge distance (fire exposure if fire attacks from more than one side)	C _{min}	[mm]		icks from mo ance shall be			ninimum

Table B2: Details of letter code on the head

Letter code on the head of cone bolt *	А	В	с	D	E	F	G	н	1	к	L	м	N	0	Ρ	R	S
Maximum thickness of fixture	5	10	15	20	25	30	35	40	45	50	55	60	65	70	80	90	100

*For $100 < t_{fix} \le 200$ there is the number 1 before the letter code;

 $200 < t_{\rm fix} \leq 300$ there is the number 2 before the letter code;

 $300 < t_{\text{fix}} \leq 400$ there is the number 3 before the letter code;

Tecfi wedge anchor AJE

Intended use

Installation parameters

Annex B 2

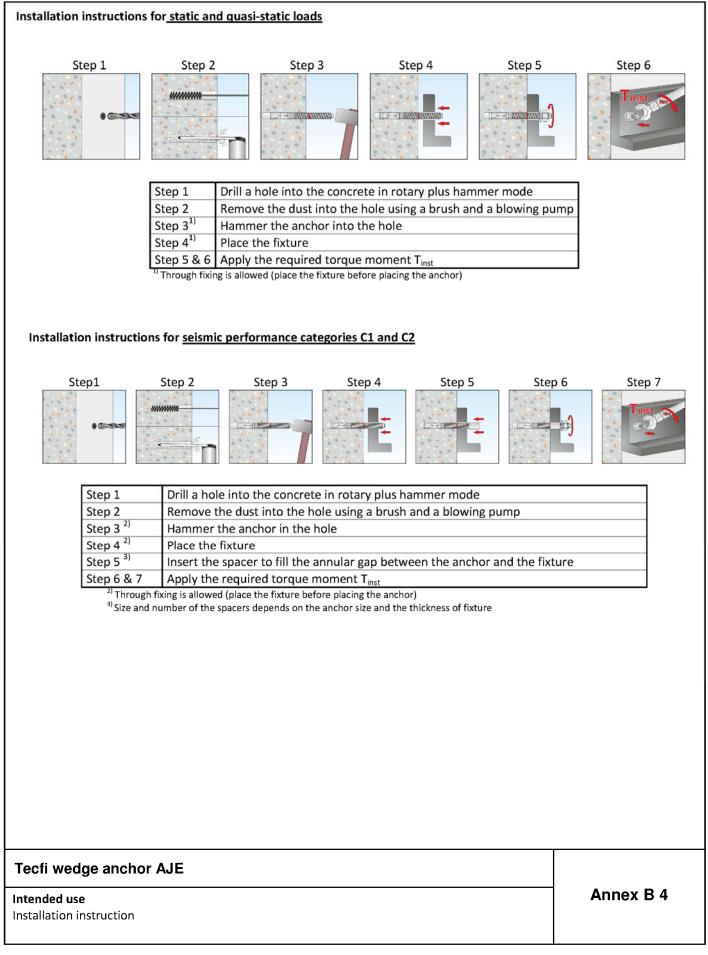
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Drill bit				
	0	AJE anchor size	Drill bit item code	
		Ø 8 (M 8)	EO 01 08 210	\neg
	· 10.	Ø 10 (M 10)	EO 01 10 210	
		Ø 12 (M 12)	EO 01 12 210	
		Ø 16 (M 16)	EO 01 16 210	
		Ø 20 (M 20)	EO 01 20 210	
Seismic Protector®			I tem code : DW 01 00 001 e: SPS	
ecfi wedge anchor A	JE			Annex E

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English translation prepared by DIBt



NRLA NRLASSENCE Characteristic resistanceNRLA NRLASSENCE NRLASSENCE (KN)162540070115NRLASSENCE NRLASSENCE VRLASSENCE VRLAN(KN)1220356095VRLASSENCE VRLAN(KN)10172445M° RLA(KN)NPD100172445M° NRLAN(RN)3060105266519Partial safety factorVMLN(-)	Anchor size			M 8	M 10	M 12	M 16	M 20
$ \begin{array}{c c c c c c } heraccheristic resistance in Gracked NR6,566C (IN) (IN) I12 (IN) I12 (IN) I12 (IN) I17 (IN) I13 (IN) I$	Steel failure					•		
$ \begin{array}{c c c c c c } \label{eq:constraints} \begin{tabular}{ c c c c } \label{eq:constraints} \begin{tabular}{ c c c c } \label{eq:constraints} \begin{tabular}{ c c c c c c } \label{eq:constraints} \begin{tabular}{ c c c c c c } \label{eq:constraints} \begin{tabular}{ c c c c c c c } \label{eq:constraints} \begin{tabular}{ c c c c c c c c c c } \label{eq:constraints} \begin{tabular}{ c c c c c c c c c c c c c c c c c c c$		N _{Rk,S,seisC1}	[kN]	16	25	40	70	115
$ \frac{1}{10} + \frac{1}{10}$	Oh e ve ete vietie ve siete e ee		[kN]	12	20	35	60	95
	Lharacteristic resistance	V _{Rk,S,seis,C1}	[kN]	NPD	10	17	24	45
$\begin{tabular}{ c c c c } \hline $V_{M6,N}$ $ [-] $$$$$$$$$$$$$$$$$$$$$$$$$$$$$$$$$		V _{Rk,S,seis,C2}	[kN]	NPD	10	17	24	45
Value of failureUnder tailureCharacteristic resistance in uncrackedNBK,p,uer[kN]7,51620Not relevantCharacteristic resistance in crackedNBK,p,er[kN]69162530Characteristic resistance under seismicNBK,p,seis,CI[kN]NPD3,212,82530Characteristic resistance under seismicNBK,p,seis,CI[kN]NPD3,215,116,1Calo/37[kN]NPD2,13,215,116,1Calo/37[kN]NPD2,13,215,116,1Calo/37[kN]NPD2,13,215,116,1Calo/37[kN] Ψ_c [c]1,4110,1Calo/37[kn] Ψ_c [c]1,41Calo/37[kn] Ψ_c [c]1,201,00Calo/37[kn] Ψ_c [c]1,201,00Calo/37[kn] Ψ_c [c]1,201,00Calo/50 Ψ_c [c]1,201,00Calor failureSign colspan="4">Sign colspan="4">SistanceCalor failure <td></td> <td>M⁰_{Rk,s}</td> <td>[Nm]</td> <td>30</td> <td>60</td> <td>105</td> <td>266</td> <td>519</td>		M ⁰ _{Rk,s}	[Nm]	30	60	105	266	519
$\begin{tabular}{ c c c c } \begin{tabular}{ c c c c c } \begin{tabular}{ c c c c c } \begin{tabular}{ c c c c c } \begin{tabular}{ c c c c } \begin{tabular}{ c c c c c } \begin{tabular}{ c c c c } \begin{tabular}{ c c c c } \begin{tabular}{ c c c c c c } \begin{tabular}{ c c c c c c c c c c c } \begin{tabular}{ c c c c c c c c c c c c c c c c c c c$	Partial safety factor	¥мs,N	[-]			1,5		
$\begin{tabular}{ c c c c c c } \hline N_{Rk,p,uer} & [KN] & 7,5 & 16 & 20 & Not relevant \\ \hline N_{Rk,p,uer} & [KN] & 6 & 9 & 16 & 25 & 30 \\ \hline N_{Rk,p,sels,C2} & [KN] & NPD & 3,2 & 12,8 & 25 & 30 \\ \hline Not relevant \\ \hline N_{Rk,p,sels,C2} & [KN] & NPD & 3,2 & 12,8 & 25 & 30 \\ \hline Not relevant \\ \hline N_{Rk,p,sels,C2} & [KN] & NPD & 3,2 & 12,8 & 25 & 30 \\ \hline Not relevant \\ \hline Not $	Pull-out failure							
$ \frac{1}{10000000000000000000000000000000000$	Characteristic resistance in <u>uncracked</u> concrete C20/25	N _{Rk,p,ucr}	[kN]	7,5	16	20	Not re	levant
$ \frac{\text{Performance category C1}{\text{C1}} \qquad N_{\text{Rk}, p, \text{sels, C2}} \qquad [\text{KN}] \qquad NPD \qquad 3,2 \qquad 12,8 \qquad 25 \qquad 30 \\ \text{Arracter istic resistance under seismic erformance category C2} \qquad N_{\text{Rk}, p, \text{sels, C2}} \qquad [\text{KN}] \qquad NPD \qquad 2,1 \qquad 3,2 \qquad 15,1 \qquad 16,1 \\ \text{Arracter istic resistance under seismic erformance category C2} \qquad N_{\text{Rk}, p, \text{sels, C2}} \qquad [\text{KN}] \qquad NPD \qquad 2,1 \qquad 3,2 \qquad 15,1 \qquad 16,1 \\ \hline 3,2 \qquad 16,1 \qquad 16,1 \\$	Characteristic resistance in <u>cracked</u> concrete C20/25	N _{Rk,p,cr}	[kN]	6	9	16	25	30
$\begin{array}{c c c c c c c } \begin{tabular}{ c c c c } \hline NRk,p,seis,C2 & [kN] & NPD & 2,1 & 3,2 & 15,1 & 16,1 \\ \hline C30/37 & & & & & & & & & & & & & & & & & & &$	performance category C1	N _{Rk,p,seis,C1}	[kN]	NPD	3,2	12,8	25	30
$\begin{array}{c c c c c c c } \hline \mbox{cd0/50} & \psi_c & [-] & \hline \mbox{c50/60} & & & & \\ \hline \mbox{c50/60} & & & \\ \hline \mbox{c60/c} & & \\ \hline \\mbox{c60/c} & & \\ \hline \mbox{c} &$	Characteristic resistance under seismi performance category C2	C N _{Rk,p,seis,C2}	[kN]	NPD	2,1	3,2	15,1	16,1
$\begin{array}{c c c c c c c } \hline \psi_c & \left[\begin{array}{c} \left[\begin{array}{c} \left[\begin{array}{c} \\ \end{array} \right] & \hline \\ \hline$	-					1,22		
nstallation safety factor γ_2 [-] $1,20$ $1,00$ Concrete cone failureEffective anchorage depth h_{ef} [mm]4555707590 k_{cr} k_{cr} $7,2$ $7,2$ $7,2$ $7,2$ k_{cr} k_{cr} $10,1$ $10,1$ $10,1$ Expansion $s_{cr,N}$ [mm] $3h_{ef}$ $1,5h_{ef}$ Edge distance $c_{cr,N}$ [mm] 200 280 300 430 400 Edge distance $c_{cr,sp}$ [mm] 100 140 150 215 200 Edge distance $c_{cr,sp}$ [mm] 100 140 150 215 200 Edge distance $c_{cr,sp}$ [mm] 100 140 150 215 200 Edge distance $k^{11} = k_3^{21}$ [-] $1,0$ $2,0$ $2,0$ Edge distance $k^{11} = k_3^{21}$ $[-]$ $1,0$ $2,0$ $2,0$ Edge distance $k^{11} = k_3^{21}$ $[-]$ $1,0$ $2,0$ $2,0$ Edge distance $k^{11} = k_3^{21}$ $[-]$ $1,0$ $2,0$ $2,0$ Edge distance $k^{11} = k_3^{21}$ $[-]$ $1,0$ $2,0$ $2,0$ Edge distance $k^{11} = k_3^{21}$ $[-]$ $1,0$ $2,0$ $3,0$ Edge distance $k^{11} = k_3^{21}$ $[-]$ $1,0$ $2,0$ $3,0$ Edge distance $k^{11} = k_3^{21}$ $[-]$ $1,0$ $2,0$ $3,0$ Edge distance $k^{$	for concreteC40/50	ψ _c	[-]			1,41		
Concrete cone failureEffective anchorage depth h_{ef} [mm]4555707590Effective anchorage depth h_{ef} [mm]4555707590Eactor 2) k_{cr} k_{cr} 10,110,110,110,1Exactor 3) $k_{cr,N}$ [mm] $3 h_{ef}$ 10,110,110,1Exactor 4) $c_{cr,N}$ [mm] $3 h_{ef}$ 10,110,110,1Exactor 5) $c_{cr,N}$ [mm] $1,5 h_{ef}$ 10,110,110,1Exactor 6) $c_{cr,Sp}$ [mm]200280300430400Edge distance $c_{cr,Sp}$ [mm]100140150215200Concrete pry-out failure $k^{11} = k_3^{21}$ [-] $1,0$ $2,0$ 2,0Concrete edge failure $k^{11} = h_{ef}$ [mm]4555707590	C50/60					1,55		
iffective anchorage depth h_{ef} [mm] 45 55 70 75 90 iffective anchorage depth k_{cr} Image: state of the state	nstallation safety factor	γ ₂	[-]		1,20		1,	00
$\frac{k_{cr}}{k_{ucr}} = \frac{k_{cr}}{10,1}$ $\frac{k_{cr}}{k_{ucr}} = \frac{7,2}{10,1}$ $\frac{k_{cr}}{10,1} = \frac{1}{1,5 h_{ef}}$ $\frac{k_{cr}}{10,1} = \frac{1}{1,5 h_{ef}}$ $\frac{k_{ucr}}{1,5 h_{ef}} = \frac{1}{1,5 h_{ef}}$ $\frac{k_{cr,N}}{1,5 h_{ef}} = \frac{1}{1,5 h_{ef}}$ $\frac{k_{cr,N}}{1,5 h_{ef}} = \frac{1}{1,0}$	Concrete cone failure							
kucr Image:	Effective anchorage depth	h _{ef}	[mm]	45	55	70	75	90
kucr 10,1 spacing scr,N [mm] $3 h_{ef}$ idge distance c_{cr,N} [mm] $1,5 h_{ef}$ splitting failure [mm] 200 280 300 430 400 spacing scr,sp [mm] 200 280 300 430 400 spacing scr,sp [mm] 100 140 150 215 200 Sepacing scr,sp [mm] 100 140 150 215 200 Scharter k ¹¹ = k ²¹ [-] $1,0$ $2,0$ $2,0$ $2,0$ Concrete edge failure k ¹¹ = k ²¹ [-] $1,0$ $2,0$ $2,0$ $2,0$ Scharter k ¹¹ = k ²¹ [-] $1,0$ $2,0$ $2,0$ $2,0$ Scharter k ¹¹ = k ²¹ [-] $1,0$ $2,0$ $2,0$ 3 Scharter k ¹¹ = k ²¹ [-] $1,0$ $2,0$ 3 3 4 Scharter k ¹¹ = k ²¹ 5 70 <td>= 2)</td> <td>k_{cr}</td> <td></td> <td></td> <td></td> <td>7,2</td> <td></td> <td></td>	= 2)	k _{cr}				7,2		
Edge distance $c_{cr,N}$ [mm] $1,5 h_{ef}$ Splitting failure Splitting failure Splitting failure $1,5 h_{ef}$ Spacing $s_{cr,sp}$ [mm] 200 280 300 430 400 Edge distance $c_{cr,sp}$ [mm] 100 140 150 215 200 Concrete pry-out failure $k^{11} = k_3^{20}$ $[-]$ $1,0$ $2,0$ $2,0$ Concrete edge failure $k^{11} = k_3^{20}$ $[-]$ $1,0$ $2,0$ $2,0$ Effective length of anchor $I_f = h_{ef}$ [mm] 45 55 70 75 90		k _{ucr}				10,1		
Splitting failure Spacing S _{cr,sp} [mm] 200 280 300 430 400 Edge distance $c_{cr,sp}$ [mm] 100 140 150 215 200 Concrete pry-out failure $c_{cr,sp}$ [mm] 100 140 150 215 200 Concrete pry-out failure $k^{11} = k_3^{-2}$ [-] 1,0 2,0 2,0 Concrete edge failure $k^{11} = k_{g}^{-2}$ [-] 1,0 2,0 2,0 Concrete edge failure $k^{11} = k_{g}^{-2}$ [-] 1,0 2,0 2,0 Concrete edge failure $k^{11} = k_{g}^{-2}$ [-] 1,0 2,0 2,0 Concrete edge failure $k^{11} = k_{g}^{-2}$ [-] 1,0 2,0 2,0 Concrete edge failure $k^{11} = k_{g}^{-2}$ [-] 1,0 2,0 2,0 Concrete edge failure $k^{10} = k_{g}^{-2}$	Spacing	S _{cr,N}	[mm]			3 h _{ef}		
Spacing $s_{cr,sp}$ [mm] 200 280 300 430 400 Edge distance $c_{cr,sp}$ [mm] 100 140 150 215 200 Concrete pry-out failure $k^{11} = k_3^{21}$ [-] $1,0$ $2,0$ 200 Concrete edge failure $k^{11} = k_3^{21}$ [-] $1,0$ $2,0$ 200 Concrete edge failure $k^{11} = k_3^{21}$ $[-]$ $1,0$ $2,0$ 200 Concrete edge failure $k^{11} = k_{10}^{21}$ Effective length of anchor $l_{f} = h_{ef}$ [mm] 45 55 70 75 90	Edge distance	C _{cr,N}	[mm]			1,5 h _{ef}		
Edge distance $c_{cr,sp}$ [mm] 100 140 150 215 200 Concrete pry-out failure $k^{11} = k_3^{21}$ [-] $1,0$ $2,0$ $2,0$ Concrete edge failure $k^{11} = k_3^{21}$ [-] $1,0$ $2,0$ $2,0$ Concrete edge failure $k^{11} = k_{gf}$ [mm] 45 55 70 75 90	Splitting failure							
Concrete pry-out failure $k^{1)} = k_3^{2)}$ [-]1,02,0Concrete edge failureEffective length of anchor $l_f = h_{ef}$ [mm]4555707590	Spacing	S _{cr,sp}	[mm]	200	280	300	430	400
k factor k ¹⁾ = k ₃ ²⁾ [-] 1,0 2,0 Concrete edge failure If = h _{ef} [mm] 45 55 70 75 90	Edge distance	C _{cr,sp}	[mm]	100	140	150	215	200
Concrete edge failureEffective length of anchor $I_f = h_{ef}$ [mm]4555707590	Concrete pry-out failure							
Effective length of anchor $I_f = h_{ef}$ [mm] 45 55 70 75 90	< factor	$k^{1} = k_3^{2}$	[-]	1	,0		2,0	
	Concrete edge failure							
Dutside diameter of anchor d _{nom} [mm] 8 10 12 16 20	Effective length of anchor	l _f = h _{ef}	[mm]	45	55	70	75	90
		d	[mm]	8	10	12	16	20

Tecfi wedge anchor AJE

Performances

for static and quasi-static action and for seismic performance categories C1 and C2

Annex C 1



Steel Failure				M 8	M 10	M 12	M 16	M 20
Characteristic	R30	F _{Rk,s,fi,30}	[kN]	0,37	0,87	1,69	3,14	4,90
resistance to	R60	F _{Rk,s,fi,60}	[kN]	0,33	0,75	1,26	2,36	3,68
tension and	R90	F _{Rk,s,fi,90}	[kN]	0,26	0,58	1,10	2,04	3,19
shear loads	R120	F _{Rk,s,fi,120}	[kN]	0,18	0,46	0,84	1,57	2,45
	R30	M ⁰ _{Rk,s,fi,30}	[Nm]	0,4	1,1	2,6	6,7	13,0
Characteristic	R60	M ⁰ _{Rk,s,fi,60}	[Nm]	0,3	1,0	2,0	5,0	9,7
bending moments	R90	M ⁰ _{Rk,s,fi,90}	[Nm]	0,3	0,7	1,7	4,3	8,4
	R120	M ⁰ _{Rk,s,fi,120}	[Nm]	0,2	0,6	1,3	3,3	6,5
Pull-out failure								
Characteristic	R 30 to R 90	N _{Rk,p,fi}	[kN]	1,5	2,25	4,00	6,25	7,5
Resistance	R 120	N _{Rk,p,fi,120}	[kN]	1,2	1,8	3,2	5,0	6,0
Concrete cone f	ailure							
Characteristic	R 30 to R 90	N _{Rk,c,fi}	[kN]	1,4	2,5	5,6	9,4	13,5
Resistance	R 120	N _{Rk,c,fi,120}	[kN]	1,1	2,0	4,5	7,5	10,8
$V_{Rk,c,fi(90)} = 0,25$	x V ⁰ _{Rk,c} (R30, R60, l value of the chai	R90) and V ⁻	$R_{k,c,fi(120)} = 0,3$ stance of a s	20 x V° _{Rk,c} (R1 single anchor	in cracked	concrete C	20/25	
V ⁰ _{Rk,c} as an initial								
V ⁰ _{Rk,c} as an initial Edge distance								
V ⁰ _{Rk,c} as an initial Edge distance	o R120	C _{cr,N}	[mm]			2 h _{ef}		
V [°] _{Rk,c} as an initial Edge distance R30 to				distance of	the anchor h		300 mm or	≥ 2 h _{ef}
V [°] _{Rk,c} as an initial Edge distance R30 to	o R120			distance of	the anchor h		300 mm or	. ≥ 2 h _{ef}

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English translation prepared by DIBt



Table C2:	Displacements
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Anchor size			M 8	M 10	M 12	M 16	M 20
Displacements under static and quasi-s	tatic <u>tensio</u> i	<u>ı</u> loads					
Service tension load in uncracked concrete C20/25 to C50/60	N _{ucr}	[kN]	3,30	6,40	7,90	16,70	23,30
Short term displacement	δ _{N0,cr}	[mm]	0,02	0,01	0,03	0,08	0,05
Long term displacement	δ _{N∞,cr}	[mm]	-	-	0,03	-	-
Service tension load in cracked concrete C20/25 to C50/60	N _{cr}	[kN]	2,40	3,60	6,40	11,90	16,70
Short term displacement	$\delta_{N0,cr}$	[mm]	0,10	0,06	0,20	0,21	0,31
Long term displacement	δ _{N∞,cr}	[mm]	1,02	0,60	0,84	1,40	0,55
Displacements under static and quasi-s	tatic <u>shear</u> l	oads					
Service shear load in cracked and uncracked concrete C20/25 to C50/60	V _{cr}	[kN]	5,7	9,5	16,7	28,6	45,2
Short term displacement	δ_{vo}	[mm]	2,0	2,0	3,0	4,0	6,0
Long term displacement	δ _{V∞}	[mm]	3,0	4,0	6,0	8,0	10,0
Displacements for Seismic performance	e category C	2					
Damage Limit State - Tension load	$\delta_{N,seis(DLS)}$	[mm]		2,39	1,74	3,34	2,48
Ultimate Limit State - Tension load	$\delta_{N,seis(ULS)}$	[mm]		10,54	15,07	14,26	10,80
Damage Limit State - Shear load	$\delta_{V,seis(DLS)}$	[mm]	NPD	3,45	3,24	4,98	4,56
Ultimate Limit State - Shear load	$\delta_{V,seis(ULS)}$	[mm]		6,21	8,37	9,00	9,64

Tecfi wedge anchor AJE

Performances Displacements Annex C 3