



Approval body for construction products and types of construction

#### **Bautechnisches Prüfamt**

An institution established by the Federal and Laender Governments



### European Technical Assessment

### **General Part**

Technical Assessment Body issuing the European Technical Assessment:

Trade name of the construction product

Product family to which the construction product belongs

Manufacturer

Manufacturing plant

This European Technical Assessment contains

This European Technical Assessment is issued in accordance with Regulation (EU) No 305/2011, on the basis of

### ETA-11/0319 of 17 July 2014

Deutsches Institut für Bautechnik

Tecfi wedge Anchor AJE

Torque controlled expansion anchor of sizes M8, M10, M12, M16 and M20 for use in concrete

Tecfi S.p.A Strada Statale Appia, Km. 193 81050 PASTORANO (CE) ITALIEN

tecfi plant

14 pages including 10 annexes which form an integral part of this assessment

Guideline for European technical approval of "Metal anchors for use in concrete", ETAG 001 Part 2: "Torque controlled expansion anchors", April 2013, used as European Assessment Document (EAD) according to Article 66 Paragraph 3 of Regulation (EU) No 305/2011.

ETA-11/0319 issued on 1 November 2012



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# European Technical Assessment ETA-11/0319

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### Specific Part

### 1 Technical description of the product

The Tecfi Wedge anchor AJE is an anchor made of galvanised steel of sizes M8, M10, M12, M16 and M20 which is placed into a drilled hole and anchored by torque-controlled expansion. The product description is given in Annex A.

### 2 Specification of the intended use in accordance with the applicable European Assessment Document

The performances given in Section 3 are only valid if the anchor is used in compliance with the specifications and conditions given in Annex B.

The verifications and assessment methods on which this European Technical Assessment is based lead to the assumption of a working life of the anchor of at least 50 years. The indications given on the working life cannot be interpreted as a guarantee given by the producer, but are to be regarded only as a means for choosing the right products in relation to the expected economically reasonable working life of the works.

### 3 Performance of the product and references to the methods used for its assessment

### 3.1 Mechanical resistance and stability (BWR 1)

| Essential characteristic                    | Performance   |
|---|---------------|
| Characteristic resistance for tension loads | See Annex C 1 |
| Characteristic resistance for shear loads   | See Annex C 1 |
| Displacements under tension loads           | See Annex C 3 |
| Displacements under shear loads             | See Annex C 3 |

### 3.2 Safety in case of fire (BWR 2)

| Essential characteristic | Performance                                     |
|--------------------------|---|
| Reaction to fire         | Anchorages satisfy requirements for<br>Class A1 |
| Resistance to fire       | See Annex C 2                                   |

### 3.3 Hygiene, health and the environment (BWR 3)

Regarding dangerous substances there may be requirements (e.g. transposed European legislation and national laws, regulations and administrative provisions) applicable to the products falling within the scope of this European Technical Assessment. In order to meet the provisions of Regulation (EU) No 305/2011, these requirements need also to be complied with, when and where they apply.



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### 3.4 Safety in use (BWR 4)

The essential characteristics regarding Safety in use are included under the Basic Works Requirement Mechanical resistance and stability.

- 3.5 Protection against noise (BWR 5) Not applicable.
- 3.6 Energy economy and heat retention (BWR 6) Not applicable.

### 3.7 Sustainable use of natural resources (BWR 7)

The sustainable use of natural resources was not investigated.

### 3.8 General aspects

The verification of durability is part of testing the essential characteristics. Durability is only ensured if the specifications of intended use according to Annex B are taken into account.

## 4 Assessment and verification of constancy of performance (AVCP) system applied, with reference to its legal base

According to Decision of the Commission of 24 June 1996 (96/582/EC) (OJ L 254 of 08.10.96 p. 62-65), the system of assessment and verification of constancy of performance (see Annex V and Article 65 Paragraph 2 to Regulation (EU) No 305/2011) given in the following table applies.

| Product   | Intended use  | Level or class | System |
|---|---|----------------|--------|
| Metal anchors for use in concrete (heavy-duty type) | For fixing and/or supporting<br>concrete structural elements or<br>heavy units such as cladding and<br>suspended ceilings | _              | 1      |

## 5 Technical details necessary for the implementation of the AVCP system, as provided for in the applicable European Assessment Dcoument

Technical details necessary for the implementation of the AVCP system are laid down in the control plan deposited at Deutsches Institut für Bautechnik.

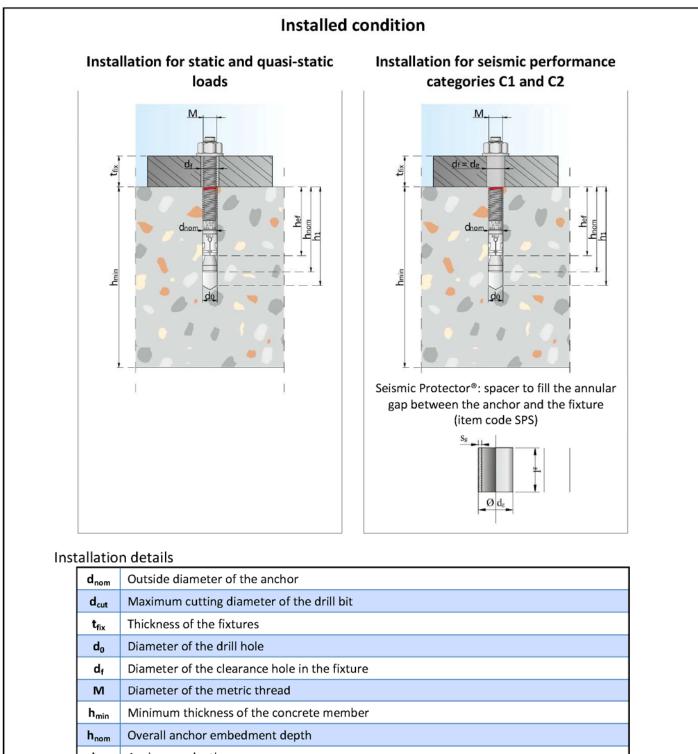
Issued in Berlin on 21 July 2014 by Deutsches Institut für Bautechnik

Uwe Bender Head of Department *beglaubigt:* Lange

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- h<sub>ef</sub> Anchorage depth
- d<sub>g</sub> Diameter of the spacer
- Ig Length of the spacer
- s<sub>g</sub> Thickness of the spacer

### Tecfi wedge anchor AJE

Product description Installed condition Annex A 1

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| Anchor types     |                         |                                      |  |   |           |
|------------------|-------------------------|--------------------------------------|--|---|-----------|
| (H)<br>Letter    |                         | Letter<br>of the<br>thickn<br>see ta | Anch<br>of fix<br>Anch<br>XX"<br>Mark<br>ember<br>Code on t<br>cone bolt<br>ess of fixt<br>ble B.2 | edment depth<br>he head<br>(maxium<br>ure), |           |
|                  | product '               | "T-AJE" (on the co<br>the clip)      | ne bolt or   | on  |           |
| AJE 01           | components:             | A                                    | JE 31 c  | components:                                 |           |
| Part             | Description             |                                      | Part   | Description                                 | 1         |
| 1                | Sleeve expansion        |                                      | 1  | Sleeve expansion                            |           |
| 2                | ISO 7089 regular washer |                                      | 2  | ISO 7093-1 large was                        | ner       |
| 3                | Hexagonal nut           |                                      | 3  | Hexagonal nut                               |           |
| 4                | Cone bolt               |                                      | 4  | Cone bolt                                   |           |
| SPS – S<br>Part  | Description             | seismic perfo                        | rmance   | categories C1 and                           | C2        |
| Tecfi wedge a    | ion                     |                                      |  |   | Annex A 2 |
| Anchor types and | i components            |                                      |  |   |           |

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### Table A1: Materials and components AJE 01 - AJE 31 components Part Description Component **Cone Bolt** Zinc plated min 5 $\mu$ m (Cr VI Free) according to ISO 4042 carbon 1 steel cone bolt, minimum tensile strength 800 N/mm<sup>2</sup> **Hexagonal nut** Zinc plated min 5 µm (Cr VI Free) according to ISO 4042 carbon 2 steel hexagonal nut DIN 934 (or ISO 4032). Washer Zinc plated min 5 $\mu$ m (Cr VI Free) according to ISO 4042 carbon 3 steel washer ISO 7089 (AJE01) or ISO 7093-1 (AJE31), hardness class HV 200. Steel sleeve Zinc plated min 5 µm (Cr VI Free) according to ISO 4042 carbon 4 steel HRB 80. SPS components Zinc plated min 5 µm (Cr VI Free) according to ISO 4042 Seismic Protector<sup>®</sup>, spacer for seismic performance 1 carbon steel spacer(s). categories C1 and C2 Assembled anchor For static and quasi-static laods AJE 01 AJE 31 For seismic performance categories C1 and C2 AJE 01 + SPS AJE 31 + SPS Tecfi wedge anchor AJE Annex A 3 **Product description** Materials

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### Specifications of intended use

### Anchorages subject to:

- Static and quasi-static loads: M8, M10, M12, M16, M20
- Seismic action for Performance Category C1 and C2: sizes M10, M12, M16, M20 with Seismic Protector® only
- Fire exposure: up to 120 minutes: M8, M10, M12, M16, M20

### **Base materials:**

- Reinforced or unreinforced normal weight concrete according to EN 206-1:2000-12.
- Strength classes C20/25 to C50/60 according to EN 206-1:2000-12. (e.g.)
- Non-cracked concrete: M8, M10, M12, M16, M20
- Cracked concrete: M8, M10, M12, M16, M20.

### Use conditions (Environmental conditions):

· Anchorages subject to dry internal conditions

### Design:

- Anchorages are designed under the responsibility of an engineer experienced in anchorages and concrete work.
- Verifiable calculation notes and drawings are prepared taking account of the loads to be anchored. The position of the anchor is indicated on the design drawings (e.g. position of the anchor relative to reinforcement or to supports, etc.).
- · Anchorages under static or quasi-static actions and under fire exposure are designed in accordance with:
  - ETAG 001, Annex C, design method A, Edition August 2010;
  - CEN TS CEN/TS 1992-4-1:2009;
- Anchorages under seismic actions are designed in accordance with:
  - EOTA Technical Report TR 045, Edition February 2013
  - Anchorages shall be positioned outside of critical regions (e.g. plastic hinges) of the concrete structure.
  - Fastenings in stand-off installation or with a grout layer are not allowed
- In case of requirements for resistance to fire exposure it must be ensured that local spalling of the concrete cover does not occur.

### Installation:

- Hole drilling by rotary plus hammer mode: M8, M10, M12, M16, M20
- Anchor installation carried out by appropriately qualified personnel and under the supervision of the person responsible for technical matters of the site.
- In case of aborted hole: new drilling at a minimum distance away of twice the depth of the aborted hole or smaller distance if the aborted hole is filled with high strength mortar and if under shear or oblique tension load it is not the direction of the load application.

### Tecfi wedge anchor AJE

Intended Use Specifications Annex B 1

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| Anchor size   |                   |      | M 8   | M 10   | M 12              | M 16  | M 20    |
|---|-------------------|------|-------|--|-------------------|-------|---------|
| Nominal drill hole diameter   | do                | [mm] | 8     | 10   | 12                | 16    | 20      |
| Maximum cutting diameter of drill bit   | $d_{cut}$         | [mm] | 8,45  | 10,45  | 12,5              | 16,5  | 20,55   |
| Maximum torque moment   | T <sub>inst</sub> | [Nm] | 20    | 45   | 60                | 110   | 200     |
| Minimum allowable spacing (even in case of fire exposure)                                 | S <sub>min</sub>  | [mm] | 80    | 65   | 75                | 130   | 170     |
| Minimum allowable edge distance   | C <sub>min</sub>  | [mm] | 80    | 80   | 90                | 130   | 200     |
| Wrench size   | SW                | [mm] | 13    | 17   | 19                | 24    | 30      |
| Overall anchor embedment depth  | h <sub>nom</sub>  | [mm] | 55    | 70   | 85                | 100   | 115     |
| Minimum thickness of concrete member  | h <sub>min</sub>  | [mm] | 100   | 110  | 140               | 170   | 200     |
| Depth of the drilled hole to deepest point  | h1                | [mm] | 65    | 85   | 105               | 120   | 135     |
| Diameter of clearance hole in the fixture   | d <sub>f</sub>    | [mm] | 9     | 12   | 14                | 18    | 22      |
| Thickness of fixture  | t <sub>fix</sub>  | [mm] | ≤ 160 | ≤ 160  | ≤ 270             | ≤ 320 |         |
| Nominal outside diameter of the spacer<br>for seismic performance categories C1<br>and C2 | dg                | [mm] | NPD   | 12   | 14                | 18    | 22      |
| Nominal length of the spacer for seismic performance categories C1 and C2                 | lg                | [mm] | NPD   | The total length of the spacer must be<br>equal to the thickness of the fixture, wit<br>tolerance of:<br>- for $t_{fix} \le 120$ [mm]: + 0 - 3 [mm]<br>- for $t_{fix} > 120$ [mm]: + 0 - 5 [mm]<br>More spacers can be used to reach the<br>total length |                   |       |         |
| Minimum edge distance (fire exposure on one side)   | C <sub>min</sub>  | [mm] |       |  | 2 h <sub>ef</sub> |       |         |
| Minimum edge distance (fire exposure if fire attacks from more than one side)             | C <sub>min</sub>  | [mm] |       | icks from mo<br>ance shall be  |                   |       | ninimum |

### Table B2: Details of letter code on the head

| Letter code on the head of cone bolt * | А | В  | с  | D  | E  | F  | G  | н  | 1  | к  | L  | м  | N  | 0  | Ρ  | R  | S   |
|--|---|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|-----|
| Maximum thickness of<br>fixture        | 5 | 10 | 15 | 20 | 25 | 30 | 35 | 40 | 45 | 50 | 55 | 60 | 65 | 70 | 80 | 90 | 100 |

\*For  $100 < t_{fix} \le 200$  there is the number 1 before the letter code;

 $200 < t_{\rm fix} \leq 300$  there is the number 2 before the letter code;

 $300 < t_{\text{fix}} \leq 400$  there is the number 3 before the letter code;

### Tecfi wedge anchor AJE

### Intended use

Installation parameters

Annex B 2

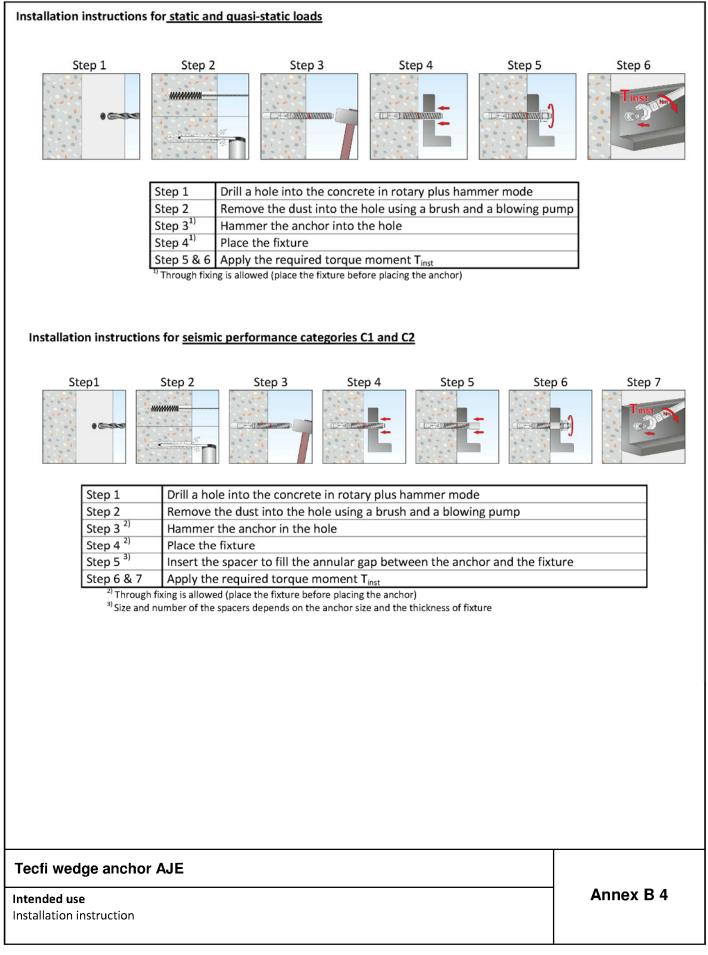
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| Drill bit           |       |                 |  |         |
|---------------------|-------|-----------------|--|---------|
|                     | 0     | AJE anchor size | Drill bit item code                        |         |
|                     |       | Ø 8 (M 8)       | EO 01 08 210                               | $\neg$  |
|                     | · 10. | Ø 10 (M 10)     | EO 01 10 210                               |         |
|                     |       | Ø 12 (M 12)     | EO 01 12 210                               |         |
|                     |       | Ø 16 (M 16)     | EO 01 16 210                               |         |
|                     |       | Ø 20 (M 20)     | EO 01 20 210                               |         |
| Seismic Protector®  |       |                 | I <b>tem code</b> : DW 01 00 001<br>e: SPS |         |
| ecfi wedge anchor A | JE    |                 |  | Annex E |

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| NRLA<br>NRLASSENCE<br>Characteristic resistanceNRLA<br>NRLASSENCE<br>NRLASSENCE<br>(KN)162540070115NRLASSENCE<br>NRLASSENCE<br>VRLASSENCE<br>VRLAN(KN)1220356095VRLASSENCE<br>VRLAN(KN)10172445M°<br>RLA(KN)NPD100172445M°<br>NRLAN(RN)3060105266519Partial safety factorVMLN(-)  | Anchor size  |                                  |      | M 8 | M 10 | M 12                | M 16   | M 20   |
|---|--|----------------------------------|------|-----|------|---------------------|--------|--------|
| $ \begin{array}{c c c c c c } heraccheristic resistance in Gracked NR6,566C (IN) (IN) I12 (IN) I12 (IN) I12 (IN) I17 (IN) I13 (IN) I$  | Steel failure  |                                  |      |     |      | •                   |        |        |
| $ \begin{array}{c c c c c c } \label{eq:constraints} \begin{tabular}{ c c c c } \label{eq:constraints} \begin{tabular}{ c c c c } \label{eq:constraints} \begin{tabular}{ c c c c c c } \label{eq:constraints} \begin{tabular}{ c c c c c c } \label{eq:constraints} \begin{tabular}{ c c c c c c c } \label{eq:constraints} \begin{tabular}{ c c c c c c c c c c } \label{eq:constraints} \begin{tabular}{ c c c c c c c c c c c c c c c c c c c$  |  | N <sub>Rk,S,seisC1</sub>         | [kN] | 16  | 25   | 40                  | 70     | 115    |
| $ \frac{1}{10} + \frac{1}{10}$  | Oh e ve ete vietie ve siete e ee   |                                  | [kN] | 12  | 20   | 35                  | 60     | 95     |
|   | Lharacteristic resistance  | V <sub>Rk,S,seis,C1</sub>        | [kN] | NPD | 10   | 17                  | 24     | 45     |
| $\begin{tabular}{ c c c c } \hline $V_{M6,N}$ $ [-] $$$$$$$$$$$$$$$$$$$$$$$$$$$$$$$$$   |  | V <sub>Rk,S,seis,C2</sub>        | [kN] | NPD | 10   | 17                  | 24     | 45     |
| Value of failureUnder tailureCharacteristic resistance in uncrackedNBK,p,uer[kN]7,51620Not relevantCharacteristic resistance in crackedNBK,p,er[kN]69162530Characteristic resistance under seismicNBK,p,seis,CI[kN]NPD3,212,82530Characteristic resistance under seismicNBK,p,seis,CI[kN]NPD3,215,116,1Calo/37[kN]NPD2,13,215,116,1Calo/37[kN]NPD2,13,215,116,1Calo/37[kN]NPD2,13,215,116,1Calo/37[kN] $\Psi_c$ [c]1,4110,1Calo/37[kn] $\Psi_c$ [c]1,41Calo/37[kn] $\Psi_c$ [c]1,201,00Calo/37[kn] $\Psi_c$ [c]1,201,00Calo/37[kn] $\Psi_c$ [c]1,201,00Calo/50 $\Psi_c$ [c]1,201,00Calor failureSign colspan="4">Sign colspan="4">SistanceCalor failure <td></td> <td>M<sup>0</sup><sub>Rk,s</sub></td> <td>[Nm]</td> <td>30</td> <td>60</td> <td>105</td> <td>266</td> <td>519</td>   |  | M <sup>0</sup> <sub>Rk,s</sub>   | [Nm] | 30  | 60   | 105                 | 266    | 519    |
| $\begin{tabular}{ c c c c } \begin{tabular}{ c c c c c } \begin{tabular}{ c c c c c } \begin{tabular}{ c c c c c } \begin{tabular}{ c c c c } \begin{tabular}{ c c c c c } \begin{tabular}{ c c c c } \begin{tabular}{ c c c c } \begin{tabular}{ c c c c c c } \begin{tabular}{ c c c c c c c c c c c } \begin{tabular}{ c c c c c c c c c c c c c c c c c c c$  | Partial safety factor  | ¥мs,N                            | [-]  |     |      | 1,5                 |        |        |
| $\begin{tabular}{ c c c c c c } \hline N_{Rk,p,uer} & [KN] & 7,5 & 16 & 20 & Not relevant \\ \hline N_{Rk,p,uer} & [KN] & 6 & 9 & 16 & 25 & 30 \\ \hline N_{Rk,p,sels,C2} & [KN] & NPD & 3,2 & 12,8 & 25 & 30 \\ \hline Not relevant \\ \hline N_{Rk,p,sels,C2} & [KN] & NPD & 3,2 & 12,8 & 25 & 30 \\ \hline Not relevant \\ \hline N_{Rk,p,sels,C2} & [KN] & NPD & 3,2 & 12,8 & 25 & 30 \\ \hline Not relevant \\ \hline Not $   | Pull-out failure   |                                  |      |     |      |                     |        |        |
| $ \frac{1}{10000000000000000000000000000000000$   | Characteristic resistance in <u>uncracked</u><br>concrete C20/25         | N <sub>Rk,p,ucr</sub>            | [kN] | 7,5 | 16   | 20                  | Not re | levant |
| $ \frac{\text{Performance category C1}{\text{C1}} \qquad N_{\text{Rk}, p, \text{sels, C2}} \qquad [\text{KN}] \qquad NPD \qquad 3,2 \qquad 12,8 \qquad 25 \qquad 30 \\ \text{Arracter istic resistance under seismic erformance category C2} \qquad N_{\text{Rk}, p, \text{sels, C2}} \qquad [\text{KN}] \qquad NPD \qquad 2,1 \qquad 3,2 \qquad 15,1 \qquad 16,1 \\ \text{Arracter istic resistance under seismic erformance category C2} \qquad N_{\text{Rk}, p, \text{sels, C2}} \qquad [\text{KN}] \qquad NPD \qquad 2,1 \qquad 3,2 \qquad 15,1 \qquad 16,1 \\ \hline 3,2 \qquad 16,1 \qquad 16,1 \\$ | Characteristic resistance in <u>cracked</u><br>concrete C20/25           | N <sub>Rk,p,cr</sub>             | [kN] | 6   | 9    | 16                  | 25     | 30     |
| $\begin{array}{c c c c c c c } \begin{tabular}{ c c c c } \hline NRk,p,seis,C2 & [kN] & NPD & 2,1 & 3,2 & 15,1 & 16,1 \\ \hline C30/37 & & & & & & & & & & & & & & & & & & &$   | performance category C1  | N <sub>Rk,p,seis,C1</sub>        | [kN] | NPD | 3,2  | 12,8                | 25     | 30     |
| $\begin{array}{c c c c c c c } \hline \mbox{cd0/50} & \psi_c & [-] & \hline \mbox{c50/60} & & & & \\ \hline \mbox{c50/60} & & & \\ \hline \mbox{c60/c} & & \\ \hline \\mbox{c60/c} & & \\ \hline \mbox{c} &$   | Characteristic resistance under seismi<br>performance category <b>C2</b> | C N <sub>Rk,p,seis,C2</sub>      | [kN] | NPD | 2,1  | 3,2                 | 15,1   | 16,1   |
| $\begin{array}{c c c c c c c } \hline \psi_c & \left[ \begin{array}{c} \left[ \begin{array}{c} \left[ \begin{array}{c} \\ \end{array} \right] & \hline \\ \hline$   | -  |                                  |      |     |      | 1,22                |        |        |
| nstallation safety factor $\gamma_2$ [-] $1,20$ $1,00$ Concrete cone failureEffective anchorage depth $h_{ef}$ [mm]4555707590 $k_{cr}$ $k_{cr}$ $7,2$ $7,2$ $7,2$ $7,2$ $k_{cr}$ $k_{cr}$ $10,1$ $10,1$ $10,1$ Expansion $s_{cr,N}$ [mm] $3h_{ef}$ $1,5h_{ef}$ Edge distance $c_{cr,N}$ [mm] $200$ $280$ $300$ $430$ $400$ Edge distance $c_{cr,sp}$ [mm] $100$ $140$ $150$ $215$ $200$ Edge distance $c_{cr,sp}$ [mm] $100$ $140$ $150$ $215$ $200$ Edge distance $c_{cr,sp}$ [mm] $100$ $140$ $150$ $215$ $200$ Edge distance $k^{11} = k_3^{21}$ [-] $1,0$ $2,0$ $2,0$ Edge distance $k^{11} = k_3^{21}$ $[-]$ $1,0$ $2,0$ $2,0$ Edge distance $k^{11} = k_3^{21}$ $[-]$ $1,0$ $2,0$ $2,0$ Edge distance $k^{11} = k_3^{21}$ $[-]$ $1,0$ $2,0$ $2,0$ Edge distance $k^{11} = k_3^{21}$ $[-]$ $1,0$ $2,0$ $2,0$ Edge distance $k^{11} = k_3^{21}$ $[-]$ $1,0$ $2,0$ $3,0$ Edge distance $k^{11} = k_3^{21}$ $[-]$ $1,0$ $2,0$ $3,0$ Edge distance $k^{11} = k_3^{21}$ $[-]$ $1,0$ $2,0$ $3,0$ Edge distance $k^{$   | for concreteC40/50   | ψ <sub>c</sub>                   | [-]  |     |      | 1,41                |        |        |
| Concrete cone failureEffective anchorage depth $h_{ef}$ [mm]4555707590Effective anchorage depth $h_{ef}$ [mm]4555707590Eactor 2) $k_{cr}$ $k_{cr}$ 10,110,110,110,1Exactor 3) $k_{cr,N}$ [mm] $3 h_{ef}$ 10,110,110,1Exactor 4) $c_{cr,N}$ [mm] $3 h_{ef}$ 10,110,110,1Exactor 5) $c_{cr,N}$ [mm] $1,5 h_{ef}$ 10,110,110,1Exactor 6) $c_{cr,Sp}$ [mm]200280300430400Edge distance $c_{cr,Sp}$ [mm]100140150215200Concrete pry-out failure $k^{11} = k_3^{21}$ [-] $1,0$ $2,0$ 2,0Concrete edge failure $k^{11} = h_{ef}$ [mm]4555707590  | C50/60   |                                  |      |     |      | 1,55                |        |        |
| iffective anchorage depth $h_{ef}$ [mm]       45       55       70       75       90         iffective anchorage depth $k_{cr}$ Image: state of the state  | nstallation safety factor  | γ <sub>2</sub>                   | [-]  |     | 1,20 |                     | 1,     | 00     |
| $\frac{k_{cr}}{k_{ucr}} = \frac{k_{cr}}{10,1}$ $\frac{k_{cr}}{k_{ucr}} = \frac{7,2}{10,1}$ $\frac{k_{cr}}{10,1} = \frac{1}{1,5 h_{ef}}$ $\frac{k_{cr}}{10,1} = \frac{1}{1,5 h_{ef}}$ $\frac{k_{ucr}}{1,5 h_{ef}} = \frac{1}{1,5 h_{ef}}$ $\frac{k_{cr,N}}{1,5 h_{ef}} = \frac{1}{1,5 h_{ef}}$ $\frac{k_{cr,N}}{1,5 h_{ef}} = \frac{1}{1,0}$   | Concrete cone failure  |                                  |      |     |      |                     |        |        |
| kucr       Image:   | Effective anchorage depth  | h <sub>ef</sub>                  | [mm] | 45  | 55   | 70                  | 75     | 90     |
| kucr       10,1         spacing       scr,N       [mm] $3 h_{ef}$ idge distance       c_{cr,N}       [mm] $1,5 h_{ef}$ splitting failure       [mm] $200$ $280$ $300$ $430$ $400$ spacing       scr,sp       [mm] $200$ $280$ $300$ $430$ $400$ spacing       scr,sp       [mm] $100$ $140$ $150$ $215$ $200$ Sepacing       scr,sp       [mm] $100$ $140$ $150$ $215$ $200$ Scharter       k <sup>11</sup> = k <sup>21</sup> [-] $1,0$ $2,0$ $2,0$ $2,0$ Concrete edge failure       k <sup>11</sup> = k <sup>21</sup> [-] $1,0$ $2,0$ $2,0$ $2,0$ Scharter       k <sup>11</sup> = k <sup>21</sup> [-] $1,0$ $2,0$ $2,0$ $2,0$ Scharter       k <sup>11</sup> = k <sup>21</sup> [-] $1,0$ $2,0$ $2,0$ $3$ Scharter       k <sup>11</sup> = k <sup>21</sup> [-] $1,0$ $2,0$ $3$ $3$ $4$ Scharter       k <sup>11</sup> = k <sup>21</sup> $5$ $70$ <td>= 2)</td> <td>k<sub>cr</sub></td> <td></td> <td></td> <td></td> <td>7,2</td> <td></td> <td></td>   | = 2)   | k <sub>cr</sub>                  |      |     |      | 7,2                 |        |        |
| Edge distance $c_{cr,N}$ [mm] $1,5 h_{ef}$ Splitting failure       Splitting failure       Splitting failure $1,5 h_{ef}$ Spacing $s_{cr,sp}$ [mm] $200$ $280$ $300$ $430$ $400$ Edge distance $c_{cr,sp}$ [mm] $100$ $140$ $150$ $215$ $200$ Concrete pry-out failure $k^{11} = k_3^{20}$ $[-]$ $1,0$ $2,0$ $2,0$ Concrete edge failure $k^{11} = k_3^{20}$ $[-]$ $1,0$ $2,0$ $2,0$ Effective length of anchor $I_f = h_{ef}$ [mm] $45$ $55$ $70$ $75$ $90$  |  | k <sub>ucr</sub>                 |      |     |      | 10,1                |        |        |
| Splitting failure         Spacing       S <sub>cr,sp</sub> [mm]       200       280       300       430       400         Edge distance $c_{cr,sp}$ [mm]       100       140       150       215       200         Concrete pry-out failure $c_{cr,sp}$ [mm]       100       140       150       215       200         Concrete pry-out failure $k^{11} = k_3^{-2}$ [-]       1,0       2,0       2,0         Concrete edge failure $k^{11} = k_{g}^{-2}$ [-]       1,0       2,0       2,0         Concrete edge failure $k^{11} = k_{g}^{-2}$ [-]       1,0       2,0       2,0         Concrete edge failure $k^{11} = k_{g}^{-2}$ [-]       1,0       2,0       2,0         Concrete edge failure $k^{11} = k_{g}^{-2}$ [-]       1,0       2,0       2,0         Concrete edge failure $k^{11} = k_{g}^{-2}$ [-]       1,0       2,0       2,0         Concrete edge failure $k^{10} = k_{g}^{-2}$   | Spacing  | S <sub>cr,N</sub>                | [mm] |     |      | 3 h <sub>ef</sub>   |        |        |
| Spacing $s_{cr,sp}$ [mm]       200       280       300       430       400         Edge distance $c_{cr,sp}$ [mm]       100       140       150       215       200         Concrete pry-out failure $k^{11} = k_3^{21}$ [-] $1,0$ $2,0$ 200         Concrete edge failure $k^{11} = k_3^{21}$ [-] $1,0$ $2,0$ 200         Concrete edge failure $k^{11} = k_3^{21}$ $[-]$ $1,0$ $2,0$ 200         Concrete edge failure $k^{11} = k_{10}^{21}$ Effective length of anchor $l_{f} = h_{ef}$ [mm] $45$ $55$ $70$ $75$ $90$  | Edge distance  | C <sub>cr,N</sub>                | [mm] |     |      | 1,5 h <sub>ef</sub> |        |        |
| Edge distance $c_{cr,sp}$ [mm]       100       140       150       215       200         Concrete pry-out failure $k^{11} = k_3^{21}$ [-] $1,0$ $2,0$ $2,0$ Concrete edge failure $k^{11} = k_3^{21}$ [-] $1,0$ $2,0$ $2,0$ Concrete edge failure $k^{11} = k_{gf}$ [mm] $45$ $55$ $70$ $75$ $90$   | Splitting failure  |                                  |      |     |      |                     |        |        |
| Concrete pry-out failure $k^{1)} = k_3^{2)}$ [-]1,02,0Concrete edge failureEffective length of anchor $l_f = h_{ef}$ [mm]4555707590   | Spacing  | S <sub>cr,sp</sub>               | [mm] | 200 | 280  | 300                 | 430    | 400    |
| k factor       k <sup>1)</sup> = k <sub>3</sub> <sup>2)</sup> [-]       1,0       2,0         Concrete edge failure       If = h <sub>ef</sub> [mm]       45       55       70       75       90  | Edge distance  | C <sub>cr,sp</sub>               | [mm] | 100 | 140  | 150                 | 215    | 200    |
| Concrete edge failureEffective length of anchor $I_f = h_{ef}$ [mm]4555707590   | Concrete pry-out failure   |                                  |      |     |      |                     |        |        |
| Effective length of anchor $I_f = h_{ef}$ [mm] 45 55 70 75 90   | < factor   | $k^{1} = k_3^{2}$                | [-]  | 1   | ,0   |                     | 2,0    |        |
|   | Concrete edge failure  |                                  |      |     |      |                     |        |        |
| Dutside diameter of anchor d <sub>nom</sub> [mm] 8 10 12 16 20  | Effective length of anchor   | l <sub>f</sub> = h <sub>ef</sub> | [mm] | 45  | 55   | 70                  | 75     | 90     |
|   |  | d                                | [mm] | 8   | 10   | 12                  | 16     | 20     |

### Tecfi wedge anchor AJE

### Performances

for static and quasi-static action and for seismic performance categories C1 and C2

Annex C 1



| Steel Failure   |  |                                       |  | M 8  | M 10         | M 12              | M 16      | M 20                  |
|---|--|---------------------------------------|--|--|--------------|-------------------|-----------|-----------------------|
|   |  |                                       |  |  |              |                   |           |                       |
| Characteristic  | R30  | F <sub>Rk,s,fi,30</sub>               | [kN]                                     | 0,37   | 0,87         | 1,69              | 3,14      | 4,90                  |
| resistance to   | R60  | F <sub>Rk,s,fi,60</sub>               | [kN]                                     | 0,33   | 0,75         | 1,26              | 2,36      | 3,68                  |
| tension and   | R90  | F <sub>Rk,s,fi,90</sub>               | [kN]                                     | 0,26   | 0,58         | 1,10              | 2,04      | 3,19                  |
| shear loads   | R120   | F <sub>Rk,s,fi,120</sub>              | [kN]                                     | 0,18   | 0,46         | 0,84              | 1,57      | 2,45                  |
|   | R30  | M <sup>0</sup> <sub>Rk,s,fi,30</sub>  | [Nm]                                     | 0,4  | 1,1          | 2,6               | 6,7       | 13,0                  |
| Characteristic  | R60  | M <sup>0</sup> <sub>Rk,s,fi,60</sub>  | [Nm]                                     | 0,3  | 1,0          | 2,0               | 5,0       | 9,7                   |
| bending<br>moments  | R90  | M <sup>0</sup> <sub>Rk,s,fi,90</sub>  | [Nm]                                     | 0,3  | 0,7          | 1,7               | 4,3       | 8,4                   |
|   | R120   | M <sup>0</sup> <sub>Rk,s,fi,120</sub> | [Nm]                                     | 0,2  | 0,6          | 1,3               | 3,3       | 6,5                   |
| Pull-out failure  |  |                                       |  |  |              |                   |           |                       |
| Characteristic  | R 30 to R 90   | N <sub>Rk,p,fi</sub>                  | [kN]                                     | 1,5  | 2,25         | 4,00              | 6,25      | 7,5                   |
| Resistance  | R 120  | N <sub>Rk,p,fi,120</sub>              | [kN]                                     | 1,2  | 1,8          | 3,2               | 5,0       | 6,0                   |
| Concrete cone f   | ailure   |                                       |  |  |              |                   |           |                       |
| Characteristic  | R 30 to R 90   | N <sub>Rk,c,fi</sub>                  | [kN]                                     | 1,4  | 2,5          | 5,6               | 9,4       | 13,5                  |
| Resistance  | R 120  | N <sub>Rk,c,fi,120</sub>              | [kN]                                     | 1,1  | 2,0          | 4,5               | 7,5       | 10,8                  |
| $V_{Rk,c,fi(90)} = 0,25$  | x V <sup>0</sup> <sub>Rk,c</sub> (R30, R60,<br>l value of the chai | R90) and V <sup>-</sup>               | $R_{k,c,fi(120)} = 0,3$<br>stance of a s | 20 x V° <sub>Rk,c</sub> (R1<br>single anchor | in cracked   | concrete C        | 20/25     |                       |
| V <sup>0</sup> <sub>Rk,c</sub> as an initial                            |  |                                       |  |  |              |                   |           |                       |
| V <sup>0</sup> <sub>Rk,c</sub> as an initial<br><b>Edge distance</b>    |  |                                       |  |  |              |                   |           |                       |
| V <sup>0</sup> <sub>Rk,c</sub> as an initial<br><b>Edge distance</b>    | o R120   | C <sub>cr,N</sub>                     | [mm]                                     |  |              | 2 h <sub>ef</sub> |           |                       |
| V <sup>°</sup> <sub>Rk,c</sub> as an initial<br>Edge distance<br>R30 to |  |                                       |  | distance of                                  | the anchor h |                   | 300 mm or | ≥ 2 h <sub>ef</sub>   |
| V <sup>°</sup> <sub>Rk,c</sub> as an initial<br>Edge distance<br>R30 to | o R120   |                                       |  | distance of                                  | the anchor h |                   | 300 mm or | . ≥ 2 h <sub>ef</sub> |

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English translation prepared by DIBt



| Table C2: | Displacements |
|-----------|---------------|
|-----------|---------------|

| Anchor size  |                        |                | M 8  | M 10  | M 12  | M 16  | M 20  |
|--|------------------------|----------------|------|-------|-------|-------|-------|
| Displacements under static and quasi-s                                   | tatic <u>tensio</u> i  | <u>ı</u> loads |      |       |       |       |       |
| Service tension load in <b>uncracked</b><br>concrete C20/25 to C50/60    | N <sub>ucr</sub>       | [kN]           | 3,30 | 6,40  | 7,90  | 16,70 | 23,30 |
| Short term displacement  | δ <sub>N0,cr</sub>     | [mm]           | 0,02 | 0,01  | 0,03  | 0,08  | 0,05  |
| Long term displacement   | δ <sub>N∞,cr</sub>     | [mm]           | -    | -     | 0,03  | -     | -     |
| Service tension load in <b>cracked</b><br>concrete C20/25 to C50/60      | N <sub>cr</sub>        | [kN]           | 2,40 | 3,60  | 6,40  | 11,90 | 16,70 |
| Short term displacement  | $\delta_{N0,cr}$       | [mm]           | 0,10 | 0,06  | 0,20  | 0,21  | 0,31  |
| Long term displacement   | δ <sub>N∞,cr</sub>     | [mm]           | 1,02 | 0,60  | 0,84  | 1,40  | 0,55  |
| Displacements under static and quasi-s                                   | tatic <u>shear</u> l   | oads           |      |       |       |       |       |
| Service shear load in cracked and<br>uncracked concrete C20/25 to C50/60 | V <sub>cr</sub>        | [kN]           | 5,7  | 9,5   | 16,7  | 28,6  | 45,2  |
| Short term displacement  | $\delta_{vo}$          | [mm]           | 2,0  | 2,0   | 3,0   | 4,0   | 6,0   |
| Long term displacement   | δ <sub>V∞</sub>        | [mm]           | 3,0  | 4,0   | 6,0   | 8,0   | 10,0  |
| Displacements for Seismic performance                                    | e category C           | 2              |      |       |       |       |       |
| Damage Limit State - Tension load  | $\delta_{N,seis(DLS)}$ | [mm]           |      | 2,39  | 1,74  | 3,34  | 2,48  |
| Ultimate Limit State - Tension load                                      | $\delta_{N,seis(ULS)}$ | [mm]           |      | 10,54 | 15,07 | 14,26 | 10,80 |
| Damage Limit State - Shear load  | $\delta_{V,seis(DLS)}$ | [mm]           | NPD  | 3,45  | 3,24  | 4,98  | 4,56  |
| Ultimate Limit State - Shear load  | $\delta_{V,seis(ULS)}$ | [mm]           |      | 6,21  | 8,37  | 9,00  | 9,64  |

### Tecfi wedge anchor AJE

Performances Displacements Annex C 3